



GENESEE RIVER BASIN GENESEE RIVER BASIN HIGHLAND PARK RESERVOIR DAM

MONROE COUNTY , NEW YORK **INVENTORY NO. N.Y. 790**



FILE COPY E



PREPARED FOR

NEW YORK DISTRICT CORPS OF ENGINEERS **AUGUST 1981**



APPROVED FOR PUBLIC RELEASE;

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| REPORT DOCUMENTATION PAGE | READ INSTRUCTIONS BEFORE COMPLETING FORM |
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| t inspection in the performing organization | |
| Based on the evaluation of the existing cond | , |
| of the Highland Park Reservoir Dam is considered nation of documents and visual observations did which are considered to constitute a hazard to he | to be good. The exami- ot reveal conditions. |

The dam impounds an offstream reservoir which has no drainage area other than the surface area of the reservoir. The reservoir has no uncontrolled spillway. However, under normal operating conditions, the dam would impound the total Probable Maximum Precipitation with maximum pool level at about three feet below the dam crest level. Therefore, the dam has adequate surcharge storage capacity.

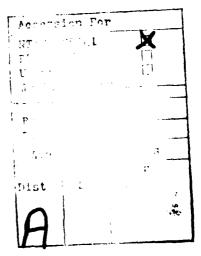
PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.



PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
HIGHLAND PARK RESERVOIR DAM
N.Y. 790
DEC I.D. NO. 40B-4450
GENESEE RIVER BASIN
MONROE COUNTY, NEW YORK

TABLE OF CONTENTS

| | PAGE NO. |
|---|----------|
| ASSESSMENT | iv |
| OVERVIEW PHOTOGRAPH | vi |
| SECTION 1: PROJECT INFORMATION | 1 |
| 1.1 GENERAL | 1 |
| 1.2 DESCRIPTION OF PROJECT | 1 |
| 1.3 PERTINENT DATA | 2 |
| SECTION 2: ENGINEERING DATA | 4 |
| 2.1 DATA AVAILABLE | 4 |
| 2.2 GEOLOGY | 4 |
| 2.3 SUBSURFACE INVESTIGATION | 4 |
| 2.4 EMBANKMENT AND APPURTENANT STRUCTURES | 4 |
| 2.5 CONSTRUCTION RECORDS | 4 |
| 2.6 OPERATING RECORDS | 5 |
| 2.7 EVALUATION OF DATA | 5 |
| SECTION 3: VISUAL INSPECTION | 6 |
| 3.1 FINDINGS | 6 |
| 3.2 EVALUATION | 6 |
| SECTION 4: OPERATION AND MAINTENANCE PROCEDURES | 7 |
| A 1 DDOCEDUDES | 7 |

TABLE OF CONTENTS (Continued)

| • | PAGE NO. |
|--|----------|
| 4.2 MAINTENANCE OF THE DAM | 7 |
| 4.3 WARNING SYSTEM IN EFFECT | 7 |
| 4.4 EVALUATION | 7 |
| SECTION 5: HYDRAULIC/HYDROLOGY | 8 |
| 5.1 DRAINAGE AREA CHARACTERISTICS | 8 |
| 5.2 ANALYSIS CRITERIA | 8 |
| 5.3 SPILLWAY CAPACITY | 8 |
| 5.4 RESERVOIR CAPACITY | 8 |
| 5.5 FLOODS OF RECORD | 8 |
| 5.6 OVERTOPPING POTENTIAL | 8 |
| 5.7 EVALUATION | 8 |
| SECTION 6: STRUCTURAL STABILITY | 9 |
| 6.1 EVALUATION OF STRUCTURAL STABILITY | 9 |
| SECTION 7: ASSESSMENT/RECOMMENDATIONS | 10 |
| 7.1 ASSESSMENT | 10 |
| 7.2 RECOMMENDATION | 10 |
| APPENDIX | |
| A. PHOTOGRAPHS | |
| B. VISUAL INSPECTION CHECKLIST | |
| C. ENGINEERING DATA CHECKLIST | |

*Not included due to lack of pertinent data.

D. HYDROLOGY AND HYDRAULIC ANALYSES*

TABLE OF CONTENTS (Continued)

- E. PLATES
- F. GEOLOGY MAP
- G. STABILITY ANALYSES*
- H. PREVIOUS INSPECTION REPORTS/AVAILABLE DATA*
- I. REFERENCES

*Not included due to lack of pertinent data.

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

Name of Dam: Highland Park Reservoir Dam

N.Y. 790

State Located: New York

County Located: Monroe

Stream: Offstream reservoir

Date of Inspection: June 3, 1981 and June 26, 1981

ASSESSMENT

Based on the evaluation of the existing conditions, the condition of the Highland Park Reservoir Dam is considered to be good. The examination of documents and visual observations did not reveal conditions which are considered to constitute a hazard to human life or property.

The dam impounds an offstream reservoir which has no drainage area other than the surface area of the reservoir. The reservoir has no uncontrolled spillway. However, under normal operating conditions, the dam would impound the total Probable Maximum Precipitation with maximum pool level at about three feet below the dam crest level. Therefore, the dam has adequate surcharge storage capacity.

The following recommendation should be implemented within three months from notification to the owner:

1. An emergency action plan should be developed, including a formal warning system to alert the downstream residents in the event of an emergency.

Assessment - Highland Park Reservoir Dam

| MINICIAN CONTRACTOR OF THE PARTY OF THE PART |
|--|
| HONWEALT |
| PROFESSIONAL AND ENGINEER No. 174584 |
| Lawrence D. Andersen |
| PEGISTERED OF PROFESSIONAL COLLEGE D. Andersen ENGINEER No. 11558 |
| MINVSYLVA |
| ANGENT POLICE |

- Justine L. Letter

Lawrence D. Andersen, P.E.

Vice President

D'Appolonia Consulting Engineers, Inc.

Pittsburgh, Pennsylvania

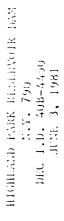
Approved by:

eol. W. M. Smith, Jr.

New York District Engineer

Date:

14 Sept1





OVERVICE

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM HIGHLAND PARK RESERVOIR DAM N.Y. 790 DEC I.D. NO. 40B-4450 GENESEE RIVER BASIN MONROE COUNTY, NEW YORK

SECTION 1: PROJECT INFORMATION

1.1 GENERAL

a. Authority

The Phase I Inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers, to fulfill the requirements of the National Dam Inspection Act, Public Law 92-367.

b. Purpose of Inspection

The purpose of the inspection was to evaluate the existing conditions of the subject dam to identify deficiencies and hazardous conditions, determine if they constitute hazards to life and property, and recommend remedial measures where necessary.

1.2 DESCRIPTION OF PROJECT

a. Dam and Appurtenances

The Highland Park Reservoir Dam is an earth embankment forming three sides of a hillside reservoir. Two of the sides have a straight alignment and are normal to each other, while the third side has a circular arc. The perimeter of the reservoir at the dam crest level is about 2,000 feet. The crest length of the embankment portion is about 1,800 feet. The maximum height is 42 feet. The embankment has a crest width which varies between 18 and 22 feet, and an inside slope of 2 horizontal to 1 vertical. The outer embankment slope varies from 2.0 to 2.3 horizontal to 1 vertical. The inside slopes are protected with riprap and the side faces are covered with grass.

The dam impounds an offstream reservoir as a storage facility for a municipal water distribution system. Inflow and outflow is controlled at two gatehouses adjacent to the embankment. One gatehouse, located at the northwest corner of the dam, contains three 24-inch-diameter pipes which connect the reservoir to the distribution system. The second gatehouse, located at the northeast corner of the dam, contains a 30-inch-diameter and two 36-inch-diameter cast iron pipes which also connect the reservoir to the distribution system. The pipes from the two gatehouses to the reservoir can be used to fill or empty the reservoir.

b. Location

The dam is located offstream in Highland Park, in Rochester, Monroe County, New York. Plate 1 illustrates the location of the dam.

c. Size Classification

The dam is classified to be of intermediate size based on its 42-foot height.

d. Hazard Classification

The dam is classified to be in the high hazard category. The reservoir is located within a heavily populated residential area of Rochester, New York.

It is estimated that failure of the dam under maximum pool level would cause loss of more than a few lives and appreciable property damage in this residential area.

e. Ownership

The dam is owned and operated by the Water Department, City of Rochester, New York. (Address: Mr. Roger C. McPhearson, Director, Water Department, City of Rochester, 10 Felix Street, Rochester, New York 14608, 716-428-7509)

f. Purpose of Dam

The dam impounds a municipal water storage reservoir.

g. Design and Construction History

The dam was designed and built around 1875.

h. Normal Operating Procedure

The reservoir is normally maintained at approximately five feet below the crest of the dam.

1.3 PERTINENT DATA

Elevations referred to in this section and subsequent sections of the report were obtained from design drawings.

a. Drainage Area (acres)

4.6

b. Discharge at Dam (cfs)

The dam has no uncontrolled spillway.

c. Elevation (USGS Datum) (feet)

| Top of | dam | 640.0 |
|--------|------|-------|
| Normal | pool | 635.0 |

d. Reservoir (acres)

| Surface are | ea at | top of | dam | 5.1 | Ĺ |
|-------------|-------|--------|------|-----|---|
| Surface are | ea at | normal | pool | 4.6 | 5 |

e. Storage Capacity (acre-feet)

Top of dam Normal pool

83 58

f. Dam

Type
Length
Height
Top width
Side slopes

Earth embankment 1,800 ± feet 42 feet 18 to 22 feet

Zoning

Inner: 2.0 to 2.3H:1V Outer: 2H:1V

Impervious core Cutoff Grout curtain No Yes Yes No

g. Primary Spillway None

h. Emergency Spillway None

i. Reservoir Drain

No formal reservoir drain; however, the reservoir can be drained through the water distribution system via the inlet/outlet facilities. Plate 2 illustrates the layout of the water distribution system associated with the reservoir. Flow through the inlet/outlet pipes are controlled by valves. The inlet/outlet pipes consist of three 24-inch pipes located at the northwest corner of the reservoir and one 30-inch and two 36-inch pipes located at the northeast end of the reservoir.

SECTION 2: ENGINEERING DATA

2.1 DATA AVAILABLE

Available information was obtained from the New York State Department of Environmental Conservation, Dam Safety Division files, and from the files of the City of Rochester, Water Department. Available information includes two design drawings.

2.2 GEOLOGY

The Highland Park Reservoir Dam lies in the Eastern Lake section of the Central Lowland Physiographic Province. This area is characterized by low relief and various glacial related features, such as moraines, lakes, and lacustrine plains.

The dam site is located on the Pinnacle Hills Moraine, which is a line of hills tending roughly east-west, and extending from the Genesee River eastward for some two miles. The moraine deposits consist largely of poorly sorted sands and gravels which can attain thicknesses of approximately 100 feet. This unit is underlain by approximately 30 feet of glacial till and lacustrine deposits. Bedrock, which is estimated to be 130 feet below the surface, consists of the Silurian Age Lockport Dolomite.

2.3 SUBSURFACE INVESTIGATION

The available information includes no reference to a subsurface investigation. Because of the age of the dam (built around 1875), it does not appear likely that any subsurface investigation was conducted.

2.4 EMBANKMENT AND APPURTENANT STRUCTURES

Plate 2 shows the typical cross section of the dam and the details of the supply pipe connections. The dam is a homogeneous embankment incorporating a centrally located clay puddle core. The embankment is shown to have a crest width of 16 feet and side slopes of 2.5 horizontal to 1 vertical on the outside face and 2 horizontal to 1 vertical on the inside face. A three-foot berm is located on the inside face about five feet below the dam crest level. As shown in Plate 2, the supply pipes are located on a timber and rubble masonry foundation equipped with masonry cutoff walls. Plate 3 shows the watermains connecting the reservoir to the distribution system.

2.5 CONSTRUCTION RECORDS

No construction records are available. Based on visual observations, the existing embankment is in general conformance with the design drawings.

2.6 OPERATING RECORDS

None available.

2.7 EVALUATION OF DATA

The information obtained from the City of Rochester, Water Department files is considered to be adequate for Phase I inspection purposes.

SECTION 3: VISUAL INSPECTION

3.1 FINDINGS

a. General

Visual inspections of the dam were conducted on June 3 and June 26, 1981. On both dates, the pool level was approximately at Elevation 635.

b. Embankment

No signs of distress, seepage, or misalignment were observed. The faces of the dam and the crest are covered with grass and were found to be adequately maintained. The top of the dam was surveyed relative to the water elevation and was found to be in the range of 0.2 to 1.5 feet above the design level.

c. Primary Spillway

The reservoir has no uncontrolled overflow spillway. The reservoir level is controlled by the inlet and outlet facilities which connect the reservoir to the water distribution system. Components of the inlet and outlet facilities were found to be in satisfactory condition.

d. Emergency Spillway

There is no emergency spillway.

e. Reservoir Drain

The inlet and outlet facilities can be used to drain the reservoir. It is reported that if the reservoir is not supplied, it would empty within one day under normal water usage conditions.

f. Downstream Channel

There is no downstream channel.

g. Reservoir

There are no visible signs of instability problems within the reservoir area.

3.2 EVALUATION

The dam was found to be in good condition. At this time, no conditions were observed that would require remedial action.

SECTION 4: OPERATION AND MAINTENANCE PROCEDURES

4.1 PROCEDURES

The reservoir is normally maintained at approximately five feet below the embankment crest level. Reservoir inflows and outflows are controlled as part of the operation of the municipal water distribution system.

4.2 MAINTENANCE OF THE DAM

The dam is maintained by the City of Rochester, Water Department. The maintenance condition of the dam is considered to be satisfactory.

4.3 WARNING SYSTEM IN EFFECT

No formal warning system exists for the dam.

4.4 EVALUATION

The maintenance condition of the dam is considered to be good. Development of an emergency action plan is advisable.

SECTION 5: HYDRAULIC/HYDROLOGY

5.1 DRAINAGE AREA CHARACTERISTICS

Highland Park Reservoir Dam has no drainage area other than the reservoir surface area.

5.2 ANALYSIS CRITERIA

As previously stated, Highland Park Reservoir Dam is classified as an intermediate dam in the high hazard category. According to the recommended criteria for evaluating emergency spillway discharge capacities, such impoundments are required to pass and/or impound the full PMF.

Since the reservoir drainage area is equal to the reservoir surface area, the runoff volume associated with the PMF event is directly equal to the rainfall volume associated with the Probable Maximum Precipitation (PMP). The 21.5-inch PMP depth to be expected in the vicinity of the impoundment will increase the reservoir elevation by 21.5 inches (1.8 feet). This 1.8-foot maximum pool level increase is well within the five feet of freeboard normally available.

5.3 SPILLWAY CAPACITY

The dam has no uncontrolled overflow spillways.

5.4 RESERVOIR CAPACITY

The dam impounds a reservoir with a storage capacity of 58 acre-feet at normal pool level (Elevation 635.0), and 83 acre-feet at the top of the dam (Elevation 640.2).

5.5 FLOODS OF RECORD

No data available.

5.6 OVERTOPPING POTENTIAL

The dam can store 100 percent of the PMF. During the PMF event, the reservoir will rise from normal pool Elevation 635.0 to a maximum level at Elevation 636.8, leaving about 3.2 feet of freeboard to the design dam crest level (Elevation 640.0).

5.7 EVALUATION

The reservoir can retain the recommended design flood of full PMF without overtopping the embankment; therefore, the surcharge storage capacity is classified to be adequate according to the recommended criteria.

SECTION 6: STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observations

As discussed in Section 3, the field observations did not reveal any signs of distress that would significantly affect the stability of the dam at this time.

b. Design and Construction Data

Available information includes two design drawings. In view of the age of the dam (built around 1875), it is not likely any materials testing or analysis was conducted in conjunction with the construction of the dam. Based on visual observations, static stability of the dam is considered to be adequate.

c. Postconstruction Changes

None reported.

d. Seismic Stability

The dam is located in Seismic Zone 3. In this zone, a horizontal acceleration of 0.1g is typically used for preliminary analysis. No data is available relative to the character of the embankment material. Therefore, the adequacy of seismic stability of the embankment could not be assessed.

SECTION 7: ASSESSMENT/RECOMMENDATIONS

7.1 ASSESSMENT

a. Safety

Visual observations indicate that the Highland Park Reservoir Dam is in good condition. No conditions were observed that would significantly affect the overall performance of the structure at this time.

It was found that the reservoir could retain the required design flood of full PMF without overtopping the embankment. Therefore, the surcharge storage capacity is classified to be adequate.

b. Adequacy of Information

Available information, in conjunction with visual observations, is considered to be sufficient to make a Phase I evaluation.

c. Need for Additional Investigations

No additional investigation is considered to be required at this time.

d. Urgency

The action recommended below should be implemented within three months from notification to the owner.

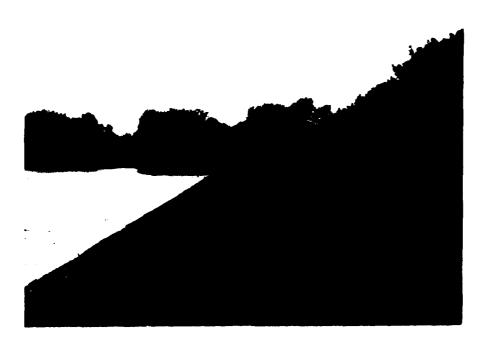
7.2 RECOMMENDATION

1. An emergency action plan should be developed, including a formal warning system to alert the downstream residents in the event of an emergency.

APPENDIX A
PHOTOGRAPHS



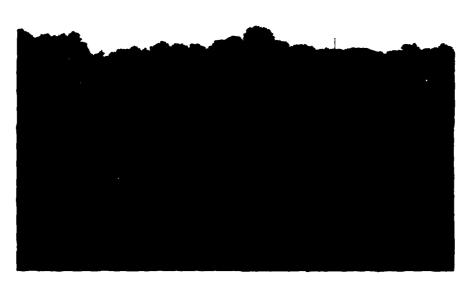
PHOTOGRAPH NO. 1
Downstream Face of the North Embankment
(looking west)



PHOTOGRAPH NO. 2
Crest of the North Embankment
(looking west)
(Note: gatehouse background)



PHOTOGRAPH NO. 3 Crest and Upstream Face (looking south)



PHOTOGRAPH NO. 4
Reservoir (looking northeast)
(Note: gatehouse background)



PHOTOGRAPH NO. 5
Reservoir (looking east)



PHOTOGRAPH NO. 6

Downstream Residential Area
(south of reservoir)

APPENDIX B

VISUAL INSPECTION CHECKLIST

APPENDIX B VISUAL INSPECTION CHECKLIST

l) Basic Data

| a. | General |
|----|---|
| | Name of Dam Highland Park Reservoir Dam |
| | Fed. I.D. # N.Y. 790 DEC Dam No. 40B-4450 |
| | River Basin Genesee River Basin |
| | Location: Highland Park, City of Rochester, Monroe County |
| | Stream Name Offstream reservoir |
| | Tributary of N/A |
| | Latitude (N) 43° 08.1' Longitude (W) 77° 36.0' |
| | Type of Dam Earth |
| | Hazard Category High |
| | Date(s) of Inspection June 3, 1981 and June 26, 1981 |
| | Weather Conditions Cloudy, Rain, Temp. 50 degrees |
| | |
| | Reservoir Level at Time of Inspection El. 635.0 ± |
| | |
| ь. | Inspection Personnel Lawrence Andersen, P.E.; James Poellot, |
| | P.E.; Bilgin Erel, P.E.; and Michael Bort |
| c. | Persons Contacted (Including Address & Phone No.) |
| | Mr. Roger C. McPhearson, Water Department, City of Rochester, |
| | 10 Felix Street, Rochester, NY 14608 (716) 428-7509 |

| | d. | Histo | ry: |
|----|-----|-------|---|
| | | Date | Constructed Around 1875 Date(s) Reconstructed N/A |
| | | Desi | gner <u>Unknown</u> |
| | | Cons | tructed byUnknown |
| | | Owne | Water Department, City of Rochester, New York |
| 2) | Emb | ankme | <u>nt</u> |
| | a. | Char | acteristics |
| | | (1) | Embankment MaterialEarth |
| | | (2) | Cutoff Type Unknown |
| | | | |
| | | (3) | Impervious Core Clay puddle. |
| | | (4) | Internal Drainage SystemNone |
| | | | |
| | | (5) | Miscellaneous |
| | b. | Cres | t |
| | | (1) | Vertical Alignment Good (0.2 to 1.5 feet above design |
| | | | dam crest level, El. 640.0) |
| | | (2) | Horizontal Alignment Good |
| | | (3) | Surface Cracks None |
| | | (4) | Miscellaneous |
| | c. | Inne | r Slope |
| | | (1) | Slope (Estimate) 2H:1V (as measured) |
| | | | |
| | | (2) | Undesirable Growth or Debris, Animal Burrows None |
| | | | |
| | | (3) | Sloughing Subsidence or Depressions None |

| (4) | Slope Protection Riprapped slope, in good condition. |
|-------|--|
| (5) | Surface Cracks or Movement at Toe None |
| Outer | r Slope |
| (1) | Slope (Estimate) 2.0 to 2.3H:1V |
| (2) | Undesirable Growth or Debris, Animal Burrows None |
| (3) | Sloughing, Subsidence or Depressions None |
| (4) | Surface Cracks or Movement at Toe None |
| (5) | Seepage None |
| (6) | External Drainage :ystem (Ditches, Trenches, Blanket) None |
| (7) | Condition Around Outlet Structure N/A |
| | Seepage Beyond Toe None |

| | (1 |) Erosion at Contact N/A |
|----|------------------|--|
| | (2 |) Seepage Along Contact N/A |
| | | |
| 3) | Draina | ge System (Embankment has no drainage system.) |
| | a. De | scription of System |
| | | |
| | b. Co | ndition of System |
| | c. Di | scharge from Drainage System |
| | | |
| 4) | Instru Piezom | mentation (Monumentation/Surveys, Observation Wells, Weirs, eters, etc.) |
| | | None |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |

| 5) | Res | Reservoir | |
|----|-----|---|--|
| | а. | Slopes Moderate slope on one side, no problems observed. | |
| | ъ. | Sedimentation No problems observed. | |
| | c. | Unusual Conditions Which Affect Dam None | |
| 6) | Are | a Downstream of Dam | |
| | a. | Downstream Hazard (No. of Homes, Highways, etc.) Residential areas of Rochester, New York, surround the embankment. | |
| | ь. | Seepage, Unusual Growth None | |
| | с. | Evidence of Movement Beyond Toe of Dam None | |
| | d. | Condition of Downstream Channel N/A | |
| 7) | | llway(s) (Including Discharge Conveyance Channel) | |
| | | mpoundment has no spillway. | |
| | а. | General | |
| | | | |
| | b. | Condition of Service Spillway | |
| | | | |

| | с. | Condition of Auxiliary Spillway N/A |
|----|-----|--|
| | a | Condition of Discharge Conveyance Channel N/A |
| | •• | donate on Discharge conveyance channel |
| 8) | Res | ervoir Drain/Outlet |
| | | Type: Pipe X Conduit Other |
| | | Material: Concrete Metal Other Three cast |
| | | iron pipes. |
| | | Two 36-inch-diameter and Size: one 24-inch-diameter Length N/A |
| | | Invert Elevations: Entrance 623.5(1) Exit N/A |
| | | Physical Condition (Describe): Not observable. |
| | | Material: |
| | | Joints: Alignment |
| | | Structural Integrity: |
| | | Hydraulic Capability: |
| | | Means of Control: Gate X Valve Uncontrolled |
| | | Operation: Operable X Inoperable Other |
| | | Present Condition (Describe): The outlets from the |
| | | reservoir are connected to the city water distribution |
| | | system and are operable. |

(1) For 24-inch-diameter outlet.

| 9) | Structural | | | | |
|----|------------|---|--|--|--|
| | a. | Concrete Surfaces The concrete screen wells appear to | | | |
| | | be in good condition. | | | |
| | | | | | |
| | | | | | |
| | b. | Structural Cracking None | | | |
| | с. | Movement - Horizontal & Vertical Alignment (Settlement) | | | |
| | | No problems observed. | | | |
| | | | | | |
| | d. | Junctions with Abutments or Embankments | | | |
| | | No problems observed. | | | |
| | | | | | |
| | | | | | |
| | e. | Drains - Foundation, Joint, Face | | | |
| | | No problems observed. | | | |
| | | | | | |
| | | | | | |
| | f. | Water Passages, Conduits, Sluices | | | |
| | | N/A | | | |
| | | | | | |
| | | | | | |
| | g. | Seepage or Leakage None observed. | | | |
| | - | | | | |
| | | | | | |

| Joints - Construction, etc. | N/A |
|-----------------------------------|-----------------|
| Foundation Not visible. | |
| Abutments N/A | |
| Control Gates <u>Good</u> | |
| Approach & Outlet Channels _ | N/A |
| Energy Dissipators (Plunge P | Pool, etc.) N/A |
| Intake Structures <u>Submer</u> g | ged |
| Stability N/A | |
| Miscellaneous | |
| | |

| Арр | ourtenant Structures (Power House, Lock, Gatehouse, Other) |
|-----|--|
| a. | Description and Condition Gatehouses in good condition. |
| | |
| • | |
| | |
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| | |
| • | |
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| • | |

APPENDIX C
ENGINEERING DATA CHECKLIST

APPENDIX C ENGINEERING DATA CHECKLIST NAME OF DAM: HIGHLAND PARK RESERVOIR DAM

AREA-CAPACITY DATA:

| | | Elevation (feet) | Surface Area (acres) | Storage Capacity (acre-feet) |
|----|-----------------------------|------------------|----------------------|------------------------------|
| 1) | Top of Dam | 640.2 | 5.1 | 83.0 |
| 2) | Design High Water | N/A | N/A_ | N/A |
| 3) | Auxiliary Spillway Crest | N/A | N/A | N/A |
| 4) | Service Spillway Crest | N/A_ | N/A | N/A |
| 5) | Normal Pool | 635.0 | 4.6 | 58.0 |

DISCHARGES

| | | Discharge (cfs) |
|----|--|-----------------|
| 1) | Average Daily | N/A |
| 2) | Spillway at Maximum High Water (Top of Dam) | N/A |
| 3) | Spillway at Design High Water | N/A |
| 4) | Principal Spillway at Dam Crest Elevation | N/A |
| 5) | Low Level Outlet | N/A |
| 6) | Total of All Facilities at Maximum High Water (Top of Dam) | N/A |
| 7) | Maximum Known Flood | Unknown |
| 8) | At Time of Inspection | N/A |

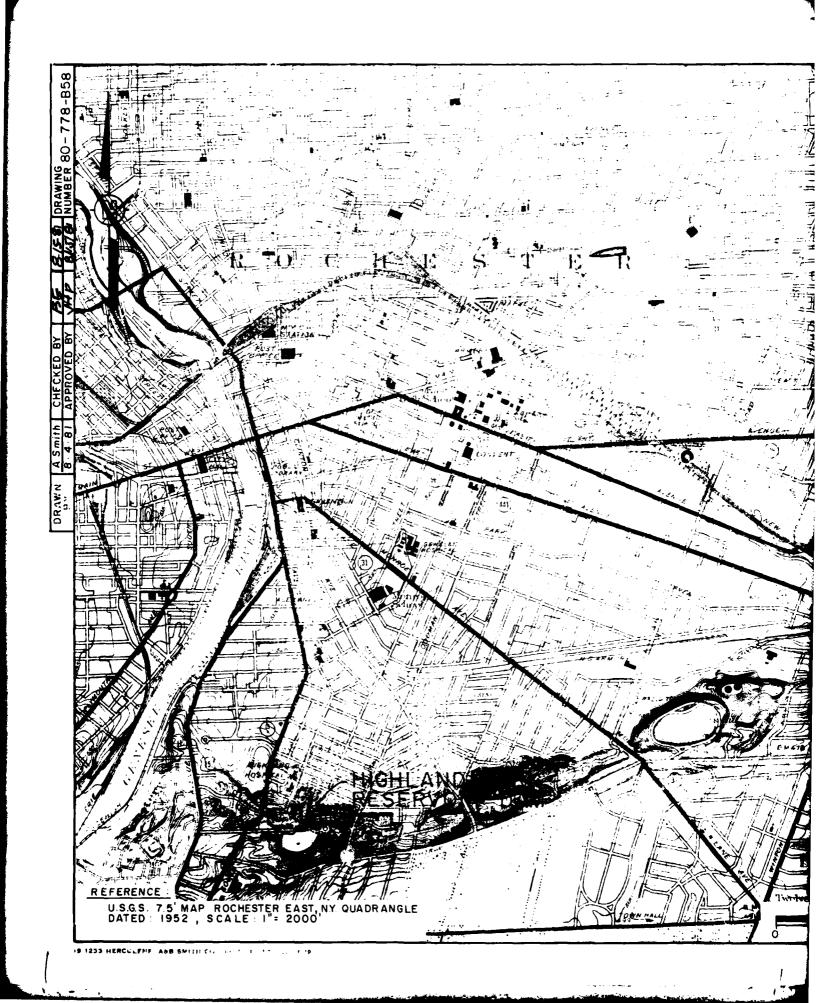
| DAM: | Highland Par | k Reservoir | Dam | | |
|---------------|--------------|-------------------------------|------------------|--|---|
| CREST ELEV | ATION: | 640.2 | | | |
| Type: _ | Earth | | | | |
| | | | | 2,060 feet | |
| Spillover: | N/A | <u> </u> | | | |
| Location: | • | | | ······································ | • |
| | | | | | |
| SPTI.I.WAY. | The reservoi | r has no sni | 11ພລບ | | |
| or the market | ine reservor | i nus no spi | . II wuy . | | |
| | SERVICE | | | AUXILIARY | |
| | | Ele | vation | | |
| | | Т | | | |
| | | | 444 | | |
| | | Type of | Control | | |
| | | Uncont | rolled | | |
| | | Contr | olled | | |
| | | | pe rds; Gate) | | |
| | | | iber | | |
| | | | Length | | |
| | | | Material | | |
| | | | ed Length | ···· | |
| | | | | | |
| | | Chute | Length | | |
| | Н | leight Betwee and Approach | | | |
| | | | r Flow) | | |

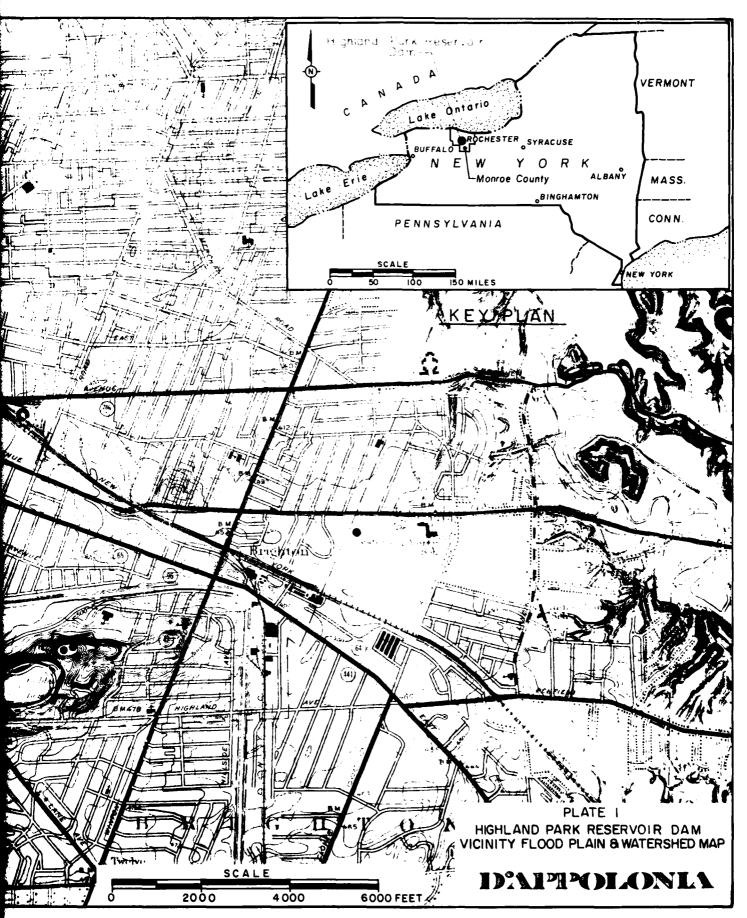
| nyarometerol | ogical Gages: |
|--------------|-----------------------------------|
| Type: | None |
| Location: | N/A |
| Records: | |
| Date | - <u>N/A</u> |
| Max. | Reading - N/A |
| FLOODWATER O | ONTROL SYSTEM: |
| Warning S | ystem: None |
| | |
| Method of | Controlled Releases (Mechanisms): |
| Th | rough water distribution system. |
| | |

| DRAINAGE AREA: Highland Park Reservoir Dam has no drainage area |
|---|
| other than the reservoir surface area. |
| DRAINAGE BASIN RUNOFF CHARACTERISTICS: |
| Land Use - Type: N/A |
| Terrain - Relief: N/A |
| Surface: N/A |
| Runoff Potential (existing or planned extensive alterations to existing surface or subsurface conditions) |
| 100 percent runoff. |
| Potential Sedimentation Problem Areas (natural or man-made; present or future) No sediment problem. |
| Potential Backwater Problem Areas for Levels at Maximum Storage Capacity Including Surcharge Storage: |
| |
| Dikes - Floodwalls (overflow and nonoverflow) - Low Reaches Along the Reservoir Perimeter: |
| Location: None |
| Elevation: |
| Reservoir: |
| Length at Maximum Pool: 800± feet |
| Length of Shoreline at Normal Pool: 2,060 feet |

APPENDIX E

PLATES





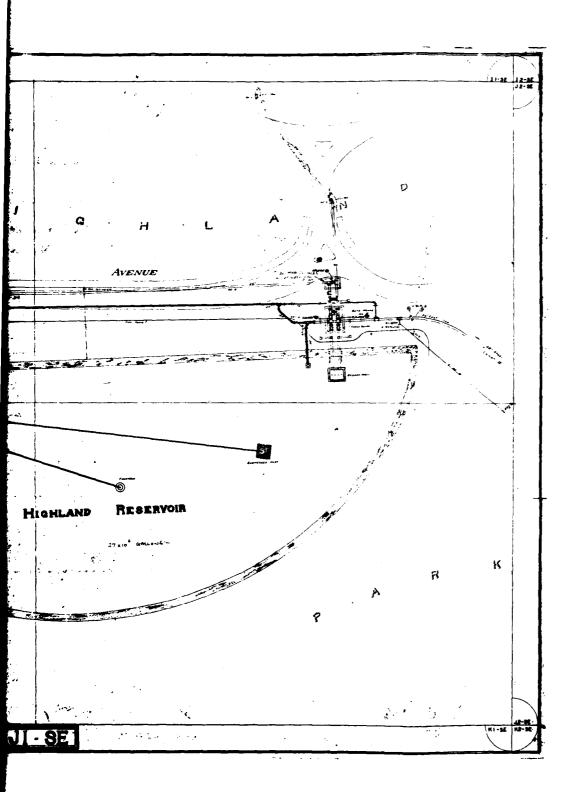
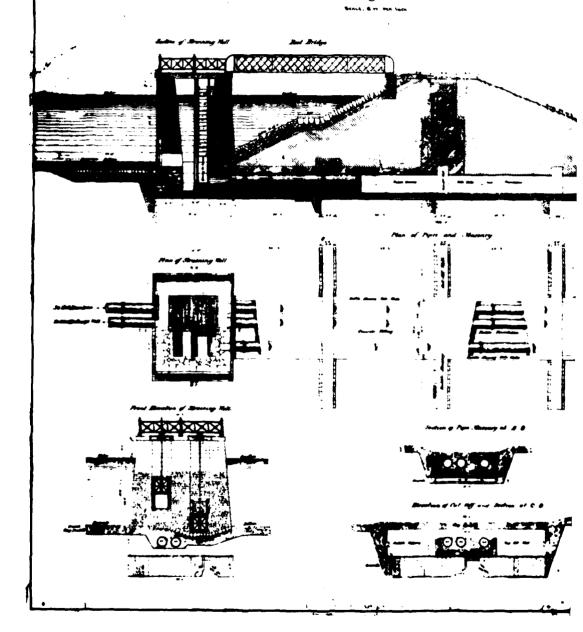


PLATE 2

DAPPOLONIA

2

Plan Shawing Manner of Luying Pipes under Bank of Distributing also Plans of Straining Well and Gate House.



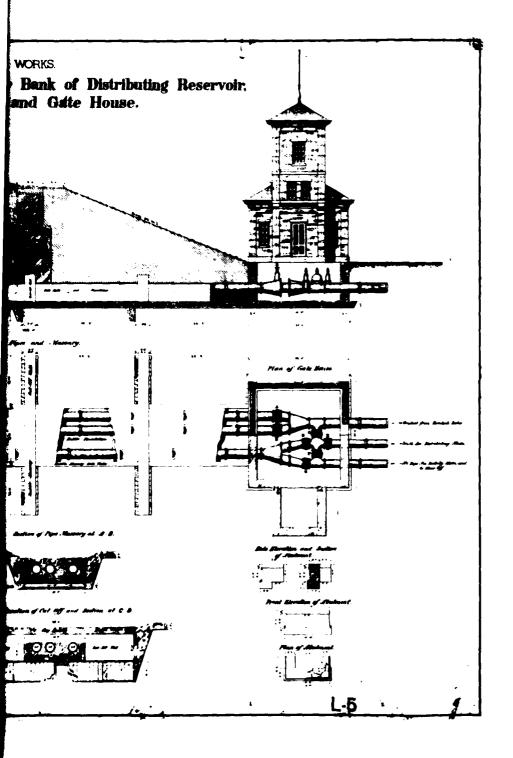


PLATE 3

DAIPPOIADNIA

NG 80-778-A - GATE HOUSE 18'-20' POOL LEVEL-SLOPE 2H: IV (REPRESENTATIVE)--GATE HOUSE NO COMMENTS

NOTE :

POOL LEVEL AT DATE OF INSPECTION: 5 FT. BELOW DAM CREST.

PLATE 4

HIGHLAND PARK RESERVOIR DAM
GENERAL PLAN
FIELD INSPECTION NOTES
FIELD INSPECTION DATE: JUNE 26,1981

D'APPOLONIA

"NOT TO SCALE"

19 1253 HERCULENE, ABB SMITH CO . PGH PA LT1930-1079

APPENDIX F

GEOLOGY MAP

Highland Reservoir Dam GEOLOGY MAP REFERENCE DAIPOIONIA GEOLOGIC MAP OF NEW YORK, FINGER LAKES SHEET DATED 1970, SCALE 1 250,000 19 1253 HERCULERE ASS SMITH CO PSH PA LT1530 1079

LEGEND

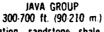
Dev

Ωį



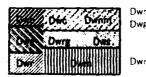
CANADAWAY GROUP 800 1200 ft (240 370 m.)

Machias Formation-shale, siltstone; Rushford Sandstone; Caneadea, Canisteo, and Hume Shales. Canaseraga Sandstone; South Wales and Dunkirk Shales. In Pennsylvania: Towanda Formation--shale sand-



Wiscoy Formation-sandstone, shale, Hanover and Pipe Creek Shales.

WEST FALLS GROUP 1100-1600 ft. (340-490 m)



Dwn Nunda Formation-sandstone, shale. West Hill and Gardeau Formations—shale, siltstone: Dwg Roricks Glen Shale; upper Beers Hill Shale; Grimes

Siltstone. tower Beers Hill Shale; Dunn Hill, Millport, and Moreland Shales.

Nunda Formation-sandstone, shale; West Hill Dwc Formation-shale, siltstone; Corning Shale

Dwnm "New Milford" Formation—sandstone, shale

Gardeau Formation-shale, siltstone; Roricks Glen

Slide Mountain Formation-sandstone, shale, con-Dw^c glomerate.

Beers Hill Shale: Grimes Siltstone: Dunn Hill. Mill-Dwm port, and Moreland Shales

SONYEA GROUP 200-1000 ft. (60-300 m.)



in west: Cashagua and Middlesex Shales. In east: Rye Point Shale: Rock Stream ("Enfield") Siltstone: Pulteney, Sawmill Creek, Johns Creek, and Montour Shales.

GENESEE GROUP AND TULLY LIMESTONE 200-1000 ft. (60-300 m.)



De West River Shale; Genundewa Limestone; Penn Yan and Geneseo Shales, all except Geneseo replaced eastwardly by Ithaca Formation—shale, siltstone and Sherburne Siltstone

Oneonta Formation—shale, sandstone Dgo Unadilla Formation—shale, siltstone Deu

Tully Limestone

SI

LOCKPORT GROUP 80-175 ft. (25-55 m.)



Oak Orchard and Penfield Dolostones, both replaced eastwardly by Sconondoa Formation-limestone. dolostone.

GEOLOGY MAP LEGEND

REFERENCE

GEOLOGIC MAP OF NEW YORK, FINGER LAKES SHEET DATED: 1970 , SCALE 1: 250,000

MACHARIA

APPENDIX I

REFERENCES

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APPENDIX I

REFERENCES

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